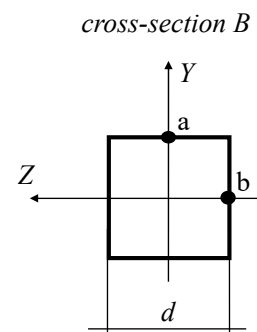
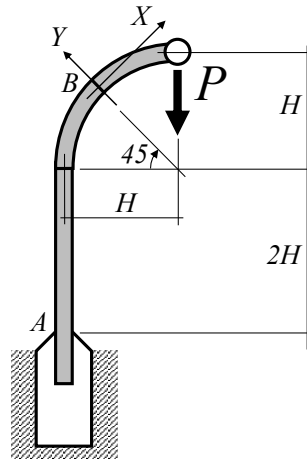
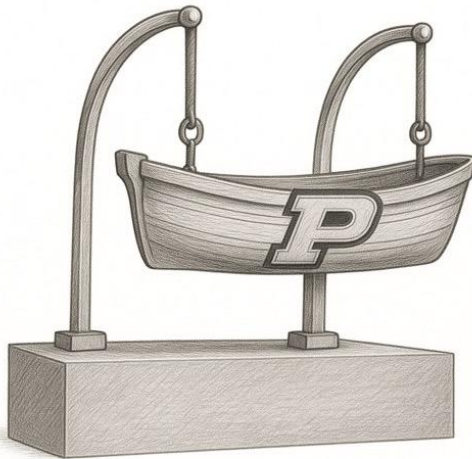
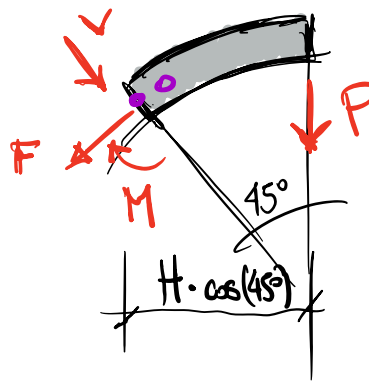
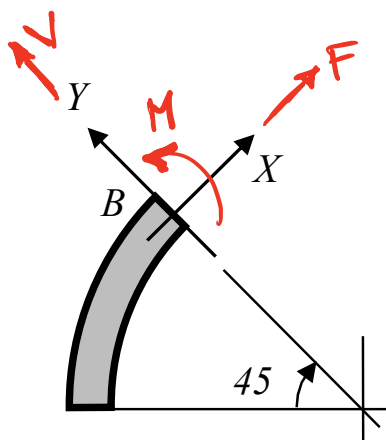


Problem ## (25 points)

A small boat is suspended by a davit-like lifting system. The boat hangs from two curved davits (arched metal arms), one on each side. The davits support the boat through vertical hanging cables ending in ring-shaped connectors. The load on each cable is equal to P . The davits are fixed to a platform at A and have a solid squared cross-section of side d .



- a) Determine the internal reactions at the cut labeled B . Classify the reaction forces as either axial, or shear forces, and the reaction moments as either bending moments, or torsion.



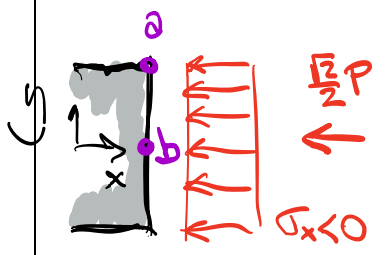
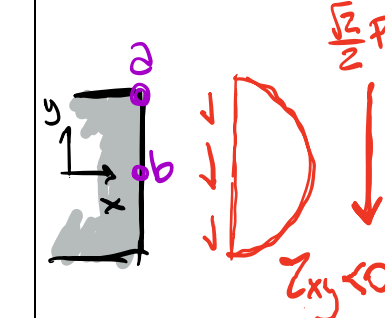
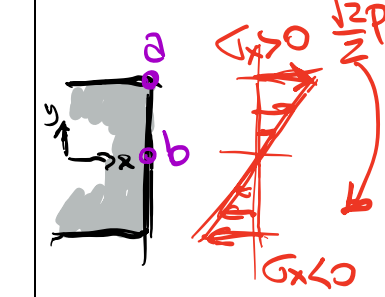
$$\begin{aligned} \oplus \rightarrow \sum F_x = 0 &= -F - \frac{\sqrt{2}}{2} P \\ \Rightarrow F &= -\frac{\sqrt{2}}{2} P \end{aligned}$$

$$\begin{aligned} \oplus \uparrow \sum F_y = 0 &= -V - \frac{\sqrt{2}}{2} P \\ \Rightarrow V &= -\frac{\sqrt{2}}{2} P \end{aligned}$$

$$\oplus \curvearrowright (\sum M)_o = 0 = M + P \cdot \frac{\sqrt{2}}{2} H \Rightarrow M = -\frac{\sqrt{2}}{2} PH$$

- b) Determine and list the stresses induced at locations “a”, and “b” on the cross section at B due to each reaction calculated in a).

Leave your answer in terms of P , H , and d .

Internal resultant	“a”	“b”
	$\sigma_x = -\frac{\sqrt{2}}{2} P \cdot \frac{1}{d^2}$	$\sigma_x = -\frac{\sqrt{2}}{2} P \cdot \frac{1}{d^2}$
	$\tau_{xy} = 0$	$\tau_{xy} = -\frac{3}{2} \frac{\sqrt{2}}{2} P \cdot \frac{1}{d^2}$
	$\sigma_x = \frac{\sqrt{2}}{2} PH \cdot \frac{d/2}{d^4/12}$	$\sigma_x = 0$

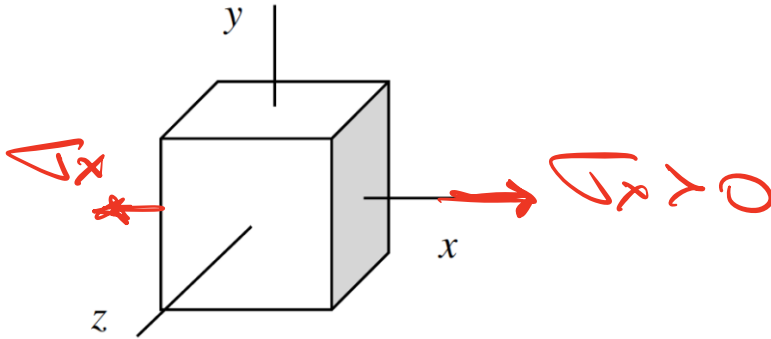
@A: $\sigma_x = -\frac{\sqrt{2}}{2} P \frac{1}{d^2} + \frac{\sqrt{2}}{2} P \frac{6H}{d^3} = \frac{\sqrt{2}}{2} \frac{P}{d^2} [-1 + \frac{6H}{d}] > 0$

$\sigma_y = \sigma_z = \tau_{xy} = \tau_{yz} = \tau_{xz} = 0$

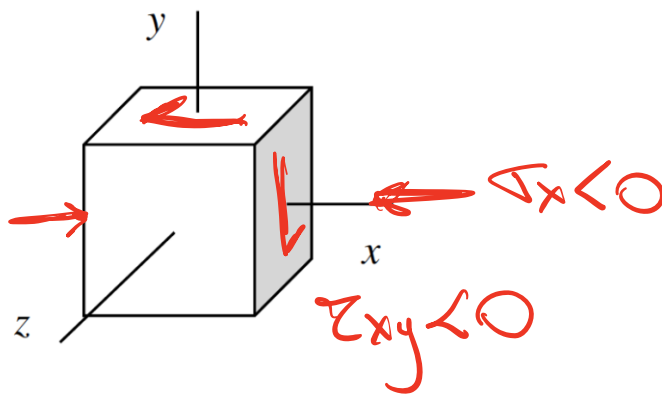
@B: $\sigma_x = -\frac{\sqrt{2}}{2} \frac{P}{d^2}$; $\tau_{xy} = -\frac{\sqrt{2}}{2} \frac{P}{d^2} \cdot \frac{3}{2}$; $\sigma_y = \sigma_z = \tau_{yz} = \tau_{xz} = 0$

- c) Draw separate stress elements for “a” and “b”, and indicate both direction, and magnitude of the stresses induced.

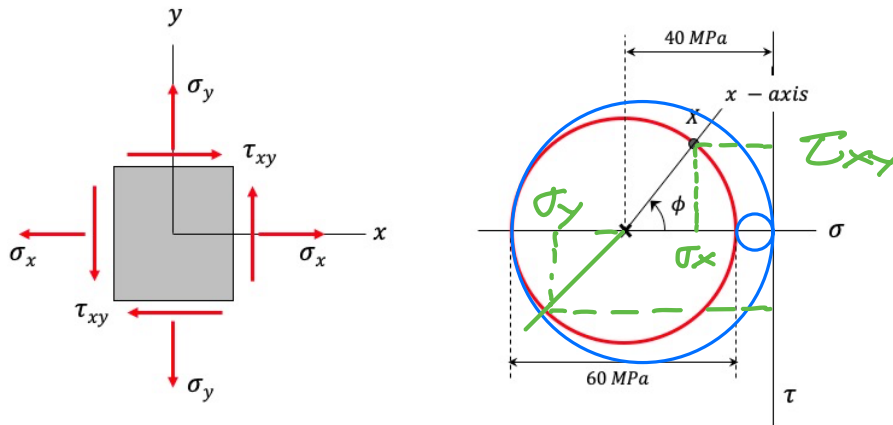
stress element at ‘a’



stress element at ‘b’



PROBLEM NO. 2 – 25 points max.



The xy -components of a state of plane stress are shown in the figure below left. The in-plane Mohr's circle for this state of stress is shown in the figure below right, where $\phi = 53.13^\circ$.

For this problem, you are asked to do the following:

- a) Determine the principal components of stress (σ_{P1} and σ_{P2}) and the maximum in-plane shear stress.

From figure:

$$\tau_{ave} = -40 \text{ MPa}$$

$$R = \frac{60}{2} = 30 \text{ MPa}$$

$$\therefore \tau_{P1} = \tau_{ave} + R = -10 \text{ MPa}$$

$$\tau_{P2} = \tau_{ave} - R = -70 \text{ MPa}$$

- b) Draw the two out-of-plane Mohr's circles on the Mohr's circle figure provided above.

- c) Determine the absolute maximum shear stress for this state of stress.

$$\begin{aligned} \tau_{max, abs} &= \text{radius of the largest Mohr's circle} \\ &= \frac{40 + 30}{2} = 35 \text{ MPa} \end{aligned}$$

- d) Determine numerical values for the xy stress components of σ_x , σ_y and τ_{xy} .

From figure:

$$\sigma_x = \sigma_{ave} + R \cos \phi = -40 + 30(0.6) = 22 \text{ MPa}$$

$$\sigma_y = \sigma_{ave} - R \cos \phi = -40 - (30)(0.6) = -58 \text{ MPa}$$

$$\tau_{xy} = -R \sin \phi = -(30)(0.8) = -24 \text{ MPa}$$

- e) What is the smallest counterclockwise angle through which the stress element must be rotated in order to expose the principal stress σ_{p1} and the principal stress σ_{p2} ?

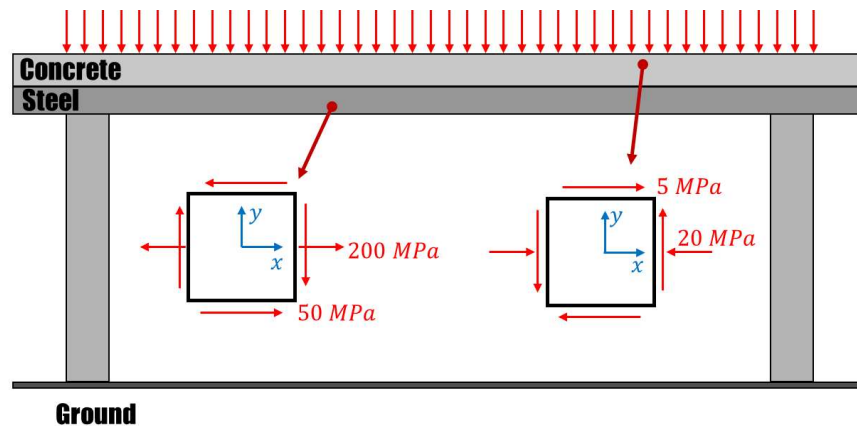
$$2\theta_{p1} = 360^\circ - \phi = 360 - 53.13^\circ = 306.87^\circ$$

$$\hookrightarrow \theta_{p1} = \frac{306.87^\circ}{2} = 153.44^\circ$$

$$2\theta_{p2} = 180^\circ - \phi = 180^\circ - 53.13^\circ = 126.87^\circ \Rightarrow \theta_{p2} = 63.44^\circ$$

PROBLEM NO. 3 – 25 points max.

A section of a steel-reinforced concrete bridge subjected to a distributed load is analyzed at two locations: one in the concrete portion and the other in the A36 steel section. For this analysis, plane-stress elements are assumed for both regions, based on the simplifying assumption that the bridge section behaves as a slender beam and that the concrete and steel exhibit comparable Poisson's ratios. The material properties are as follows: A36 steel has a yield strength of 250 MPa, while the concrete has an ultimate compressive strength of 30 MPa and an ultimate tensile strength of 5 MPa.



- a) Determine if the A36 steel section will fail according to the *Maximum Shear Stress Theory*.

$$\tau_{avg} = \frac{200 \text{ MPa}}{2} = 100 \text{ MPa}$$

(6 pts)

$$R = \sqrt{\left(\frac{200}{2}\right)^2 + (50)^2} = 111.8 \text{ MPa}$$

$$\tau_{P1} = \tau_{avg} + R = 211.8 \text{ MPa}$$

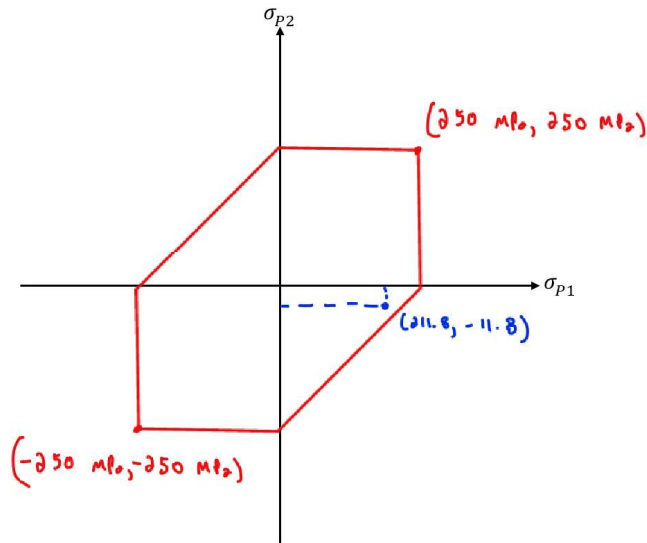
$$\tau_{P2} = \tau_{avg} - R = -11.8 \text{ MPa}$$

$$\tau_{max}^{Abs} = 111.8 \text{ MPa} < \frac{\sigma_y}{2} = \frac{250 \text{ MPa}}{2} = 125 \text{ MPa}$$

$$111.8 \text{ MPa} < 125 \text{ MPa}$$

won't fail according to MSS.

- b) Draw the failure boundary for the maximum shear stress theory. Indicate the values at the vertices of the failure boundary and indicate with a dot the state of stress in the A36 steel section.



- c) Determine the Factor of Safety for the A36 steel section.

$$F.S. = \frac{\frac{\sigma_v}{2}}{\tau_{max}} = \frac{125}{111.8} = 1.118$$

- d) Determine if the concrete section will fail according to the Mohr's failure criteria.

$$\sigma_{avg} = \frac{-20}{2} = -10 \text{ MPa}$$

$$R = \sqrt{\left(\frac{-20}{2}\right)^2 + (5)^2} = 11.18$$

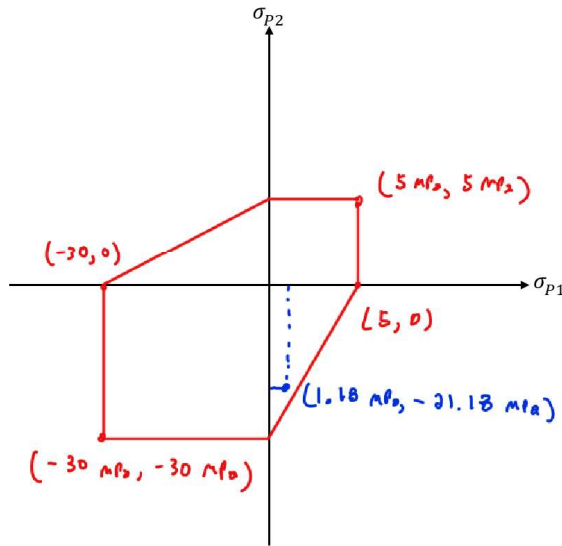
$$\sigma_{P1} = \sigma_{avg} + R = 1.18 \text{ MPa}$$

$$\sigma_{P2} = \sigma_{avg} - R = -21.18 \text{ MPa}$$

$$\frac{\sigma_{I1}}{\sigma_{UT}} - \frac{\sigma_{P2}}{\sigma_{UC}} = \frac{1.18}{5} - \frac{(-21.18)}{30} = 0.236 + 0.706 = 0.942 < 1$$

won't fail according to Mohr's failure criteria!

- e) Draw the failure boundary for the Mohr's failure criteria. Indicate the values at the vertices of the failure boundary and indicate with a dot the state of stress in the concrete section.



- f) Determine the Factor of Safety for the concrete section (use Mohr's failure criteria).

$$F.S. = \frac{1}{\frac{\sigma_{1c}}{\sigma_{0c}} - \frac{\sigma_{p2}}{\sigma_{0c}}} = \frac{1}{0.942} = 1.062$$

(2 pts)

- g) Indicate what is the most critical region of the bridge (Steel section or Concrete section).

$$F.S._{concrete} = 1.062 < F.S._{steel} = 1.113$$

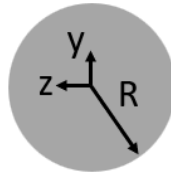
(1 pt)

concrete is the most critical

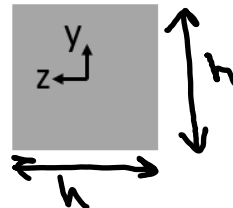
PROBLEM NO. 4

Part 4A (4 points)

Two cantilevers have the same length L and have a transverse load applied at the end. The two beams have the same cross-sectional area A , but one has a circular cross-section and the other has a square cross-section.



A



B

4A.i Which beam has a larger maximum shear stress?

$$\tau_{max,A} > \tau_{max,B}$$

$$\tau_{max,A} = \tau_{max,B}$$

$$\tau_{max,A} < \tau_{max,B}$$

$$\tau_{max, circle} = \frac{4V}{3A} = \tau_{max, A}$$

$$\tau_{max, rectangle} = \frac{3V}{2A} = \tau_{max, B}$$

4A.ii Which beam has a larger maximum flexural stress?

$$\sigma_{max,A} > \sigma_{max,B}$$

$$\sigma_{max,A} = \sigma_{max,B}$$

$$\sigma_{max,A} < \sigma_{max,B}$$

$$\pi R^2 = h^2$$

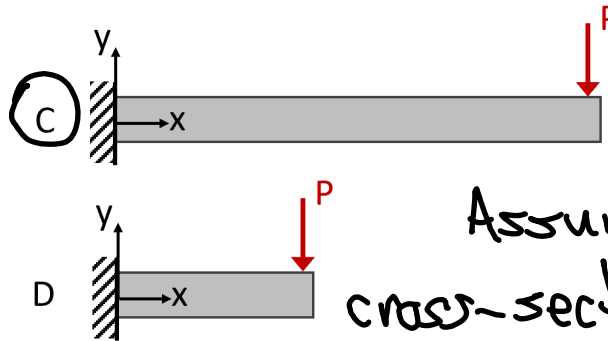
$$h = \sqrt{\pi} R$$

$$\begin{aligned} \sigma_{max,A} &= \frac{|M|R}{I_p} \\ &= \frac{4|M|}{\pi R^3} \\ &= 1.27 \frac{|M|}{R^3} \end{aligned}$$

$$\begin{aligned} \sigma_{max,B} &= \frac{|M|(\frac{h}{2})}{\frac{h^4}{12}} \\ &= \frac{6|M|}{h^3} \\ &= \frac{6|M|}{\pi^{3/2} R^3} = 1.08 \frac{|M|}{R^3} \end{aligned}$$

Part 4B (1 point)

Two cantilevers have a transverse load applied at the end. Both have the same square cross-sections, but they have different lengths. For which beam is the equation for shear stress ($\tau = VA^*\bar{y}^*/It$) the most accurate? (circle one)



Assumption of plane cross-sections is most accurate for long thin beams.

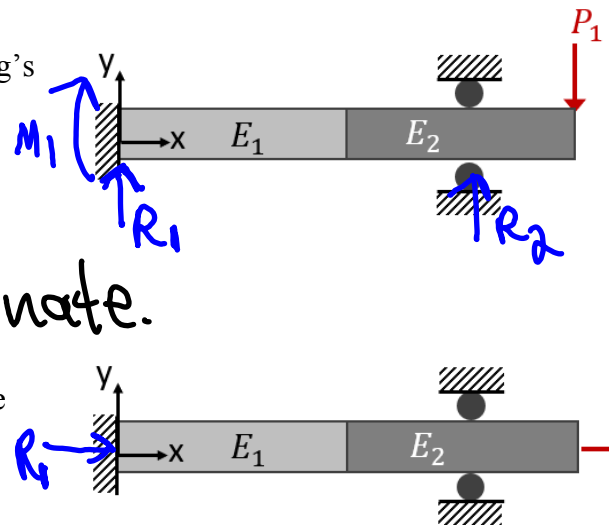
Part 4C (4 points)

A beam is made of two materials that have different Young's moduli E_1 and E_2 .

4C.i When a transverse load is applied to the beam (P_1), the external reaction(s) depend on the value of the Young's modulus E_2 .

☒ TRUE ☐ FALSE

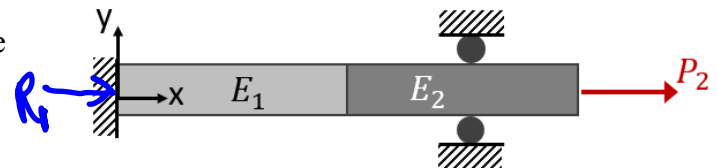
3 unknowns M_1
2 eqns
Indeterminate.



4C.ii When an axial load is applied to the beam (P_2), the external reaction(s) depend on the value of the Young's modulus E_2 .

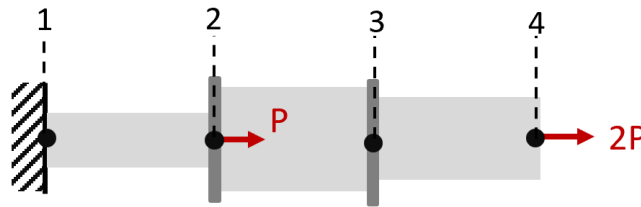
TRUE ☒ FALSE

1 unknown
1 eqns
Determinate.



Part 4D (6 points)

The axial system below will be analyzed using the finite element method using 4 nodes.



4D.i Fill in the missing terms of the stiffness matrix prior to the enforcement of the boundary conditions:

$$[K] = \frac{EA}{3L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 4 & -3 & 0 \\ 0 & -3 & 5 & -2 \\ 0 & 0 & -2 & 2 \end{bmatrix}$$

$$\begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1+k_2 & -k_2 & 0 \\ 0 & -k_2 & k_2+k_3 & -k_3 \\ 0 & 0 & -k_3 & k_3 \end{bmatrix} \quad \begin{matrix} k_1=1 \\ k_2=3 \\ k_3=2 \end{matrix}$$

4D.ii Fill in the force vector prior to the enforcement of the boundary conditions:

$$\{F\} = \begin{bmatrix} R_1 \\ P \\ 0 \\ 2P \end{bmatrix}$$

R_1 is a reaction
(e.g. F_x , P_A can also be used).

4D.iii If the number of nodes was increased to 8, the accuracy of the finite element calculations would improve (circle one).

TRUE

☒ FALSE

Assumption behind FEM is $k = \frac{(EA)_{avg}}{L}$. When E and A are uniform within the elements, the solution is already exact and will not improve by adding more nodes.

Part 4E (4 points)

A column is long and slender so that it undergoes Euler buckling. The column is pinned on both ends and has a diameter of D , a length of L , and a Young's modulus of E .

4E.i If the column is fixed on both sides, how does the critical buckling force change? (circle one)

$P_{cr, fixed-fixed} = 4P_{cr, pinned-pinned}$

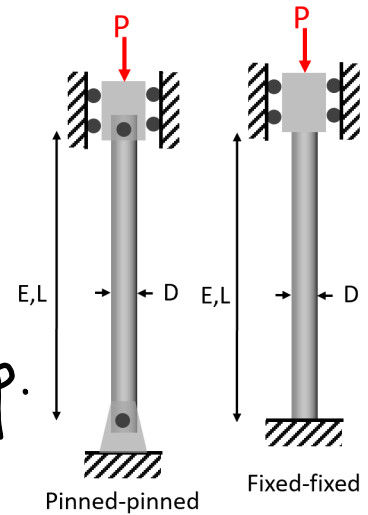
$P_{cr, fixed-fixed} = 2P_{cr, pinned-pinned}$

$P_{cr, fixed-fixed} = P_{cr, pinned-pinned}/2$

$P_{cr, fixed-fixed} = P_{cr, pinned-pinned}/4$

None of the above

$$P_{cr, f-f} = \frac{\pi^2 EI}{(0.5L)^2} = \frac{4\pi^2 EI}{L^2} = 4 P_{cr, p-p}$$



4E.ii If the Young's modulus of the pinned-pinned column is doubled to $2E$, how does the critical buckling force change? (circle one)

$P_{cr, 2E} = 4P_{cr, E}$

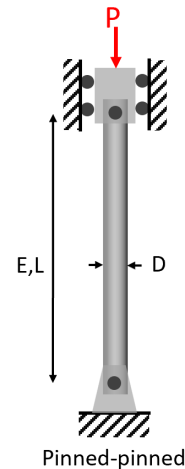
$P_{cr, 2E} = 2P_{cr, E}$

$P_{cr, 2E} = P_{cr, E}/2$

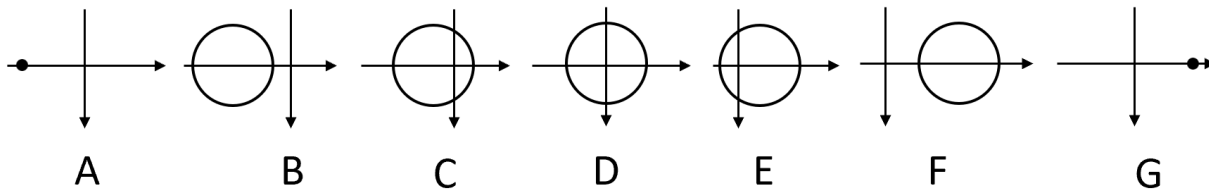
$P_{cr, 2E} = P_{cr, E}/4$

None of the above

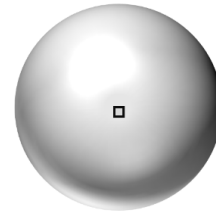
$$P_{cr, 2E} = \frac{\pi^2 (2E) I}{L^2} = 2 \frac{\pi^2 EI}{L^2} = 2 P_{cr, E}$$



Part 4F (6 points)



4F.a A spherical pressure vessel has positive pressure inside and is made of a brittle material with $\sigma_{UC} > \sigma_{UT}$.



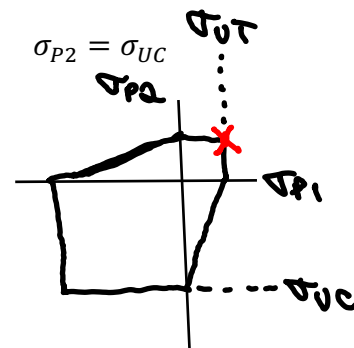
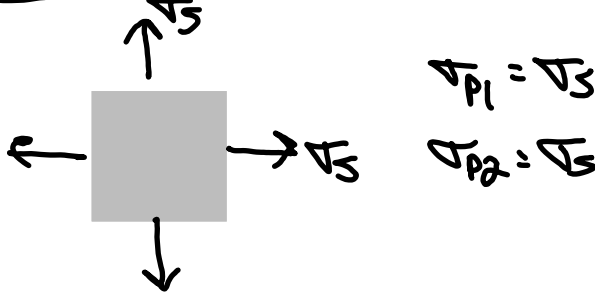
4F.a(i) Circle the correct Mohr's circle for this stress state:

A B C D E F **G**

4F.a(ii) What is the condition for failure? (circle one)

$\sigma_{P1} = \sigma_{UT}$

$\frac{\sigma_{P1}}{\sigma_{UT}} = \frac{\sigma_{P2}}{\sigma_{UC}} + 1$



4F.b A circular bar has a tensile force and a torque applied and is made of a brittle material with $\sigma_{UC} > \sigma_{UT}$.



4F.b(i) Circle the correct Mohr's circle for this stress state:

A B C D **E** F G

4F.b(ii) What is the condition for failure? (circle one)

$\sigma_{P1} = \sigma_{UT}$

$\frac{\sigma_{P1}}{\sigma_{UT}} = \frac{\sigma_{P2}}{\sigma_{UC}} + 1$

