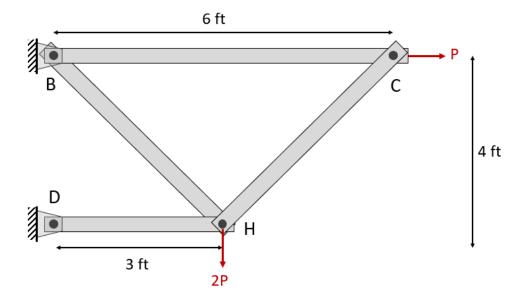
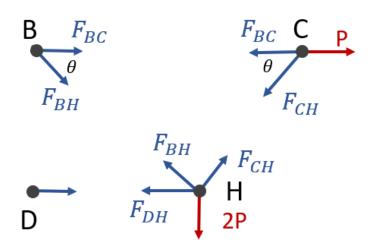
Problem 2.1 (10 points)



Each of the bars of truss have a cross-sectional area of 1.5 in² and an axial failure stress $\sigma_f = 30 \ ksi$.

- (a) Draw the free body diagrams.
- (b) Determine the forces in each member as a function of the applied force of P.
- (c) Determine the maximum magnitude P of the loads that can be applied to the truss that meets factor of safety FS = 2.

(a)



(b)
$$(\Sigma F_y)_C = 0 = -F_{CH} \sin(\theta) \rightarrow F_{CH} = 0$$

$$(\Sigma F_x)_C = 0 = P - F_{BC} - F_{CH}\sin(\theta) \to F_{BC} = P$$

$$(\Sigma F_y)_H = 0 = F_{BH} \sin(\theta) + F_{CH} \sin(\theta) - 2P$$

$$\rightarrow F_{BH}\left(\frac{4}{5}\right) = 2P \rightarrow F_{BH} = \frac{5}{2}P$$

$$(\Sigma F_x)_H = 0 = F_{CH}\cos(\theta) - F_{BH}\cos(\theta) - F_{DH}$$

$$\rightarrow F_{DH} = -F_{BH} \left(\frac{3}{5}\right) = -\left(\frac{3}{5}\right) \left(\frac{5}{2}\right) P = -\frac{3}{2} P$$

(c)
$$\sigma_{BC} = \frac{P}{A}$$

$$\sigma_{BH} = \frac{5P}{2A}$$

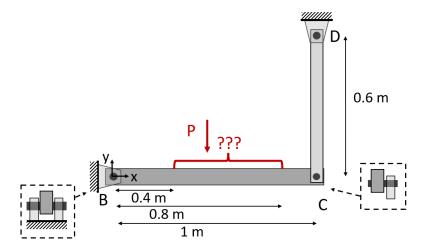
$$\sigma_{DH} = -\frac{3P}{2A}$$

A is the same for all members \rightarrow largest stress is in member BH.

$$FS = \frac{\sigma_f}{\sigma_{BH}}$$

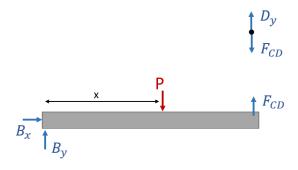
$$\sigma_{BH} = \frac{\sigma_f}{FS} = \frac{5P}{2A} \rightarrow P = \frac{2\sigma_f A}{5FS} = \frac{2(30,000 \ psi)(1.5 \ in^2)}{5*2} = 9000 \ lbs = 9 \ ksi$$

Problem 2.2 (10 points)



BCD is a shelf that has a double-sided pin at B and a single-sided pin at C. The ultimate stresses of the pins are both 60 MPa. A load of P = 1 kN will be placed on the BC section of the shelf somewhere between 0.4 m and 0.8 m from B.

- (a) Determine the minimum diameter of the pin at B to achieve a factor of safety of at least 3.0 for all possible loading conditions.
- (b) Determine the minimum diameter of the pin at C to achieve a factor of safety of at least 3.0 for all possible loading conditions.



$$(\Sigma F_y)_D = 0 = D_y - F_{CD} \to F_{CD} = D_y$$

$$(\Sigma M)_B = 0 = -Px + F_{CD}(1 m) \to F_{CD} = Px \qquad 0.4 \le x \le 0.8$$

$$(\Sigma F_y)_{BC} = 0 = B_y - P + F_{CD} \to B_y = P - F_{CD} = P(1 - x) \qquad 0.4 \le x \le 0.8$$

$$(\Sigma F_x)_{BC} = 0 = B_x$$

(a) Pin at B:

$$V_B = |B| = \sqrt{B_x^2 + B_y^2} = B_y = P(1 - x)$$

 B_y is largest when x = 0.4.

$$|B| = P(1 - 0.4) = 0.6P$$

Double shear:

$$\tau_B = \frac{|B|}{2A} = \frac{0.6P}{2\left(\pi \left(\frac{d_B}{2}\right)^2\right)}$$

$$FS = \frac{\tau_U}{\tau_B} \to \tau_B = \frac{\tau_U}{FS}$$

$$\frac{\tau_U}{FS} = \frac{0.6P}{2\left(\pi\left(\frac{d_B}{2}\right)^2\right)}$$

$$d_B = \sqrt{\frac{0.6P(2)FS}{\tau_U \pi}} = \sqrt{\frac{0.6(1000 \, N)(2)(3)}{\pi (60*10^6)}} = 0.0044 \, m = 4.4 \, mm$$

(b) Pin at C:

$$V_C = F_{CD} = Px$$

 F_{CD} is largest when x = 0.8 m

$$F_{CD}=0.8P$$

$$\tau_C = \frac{F_{CD}}{A} = \frac{0.8P}{\pi \left(\frac{d_C}{2}\right)^2}$$

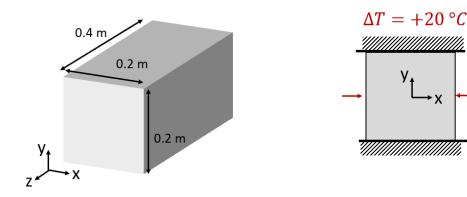
$$FS = \frac{\tau_U}{\tau_C} \rightarrow \tau_C = \frac{\tau_U}{FS}$$

$$\frac{\tau_U}{FS} = \frac{0.8P}{\pi \left(\frac{d_C}{2}\right)^2}$$

$$d_C = \sqrt{\frac{0.8P(4)FS}{\tau_U \pi}} = \sqrt{\frac{0.8(1000 \, N)(4)(3)}{\pi (60*10^6)}} = 0.0071 \, m = 7.1 \, mm$$

400 kN

Problem 2.3 (10 points)



A block of materials has initial dimensions of 0.2, 0.2, and 0.4 m in the x, y, and z directions when no stress is applied. The block is constrained in the y-direction and has a force of 400 kN applied in the x-direction and has no constraint in the z-direction. The temperature of the block is increased by 20 °C ($\Delta T = 20$ °C). The block is made of epoxy with a Young's modulus of 4 GPa, a Poisson's ratio of 0.45, and a thermal expansion coefficient of 40 x 10⁻⁶ °C⁻¹.

- (a) Determine the stress applied on the fixed surfaces in the y-direction when the load is applied and the temperature is increased.
- (b) Determine the length of the block in the z direction when the load is applied and the temperature is increased.

$$\epsilon_x = ?$$

$$\sigma_x = -\frac{400,000N}{0.2*0.4} = 5*10^6 Pa = 5 MPa$$
 $\epsilon_y = 0$

$$\sigma_y = ?$$
 $\epsilon_z = ?$

$$\sigma_z = 0$$

→3 known and 3 unknown

$$\epsilon_{x} = \frac{1}{E} \left(\sigma_{x} - \nu \left(\sigma_{y} + \sigma_{z}^{0} \right) \right) + \alpha \Delta T$$

$$\epsilon_{y} = \frac{1}{E} \left(\sigma_{y} - \nu \left(\sigma_{x} + \sigma_{z}^{0} \right) \right) + \alpha \Delta T$$

$$\epsilon_{z} = \frac{1}{E} \left(\sigma_{z}^{0} - \nu \left(\sigma_{x} + \sigma_{y} \right) \right) + \alpha \Delta T$$

(a) Use equation for ϵ_{ν} because we know all values in that equation except σ_{ν} .

$$0 = (\sigma_y - \nu \sigma_x) + E\alpha \Delta T$$

$$\sigma_y = \nu \sigma_x - E\alpha \Delta T = 0.45(-5*10^6) - 4*10^9(40*10^{-6})*20$$

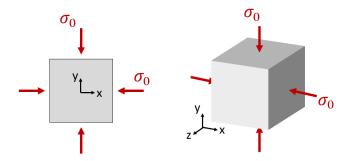
$$\sigma_y = -2.25 MPa - 3.2 MPa = -5.45 MPa$$

(b) Use equation for ϵ_z because the question is asking about z deformation.

$$\begin{split} \epsilon_z &= \frac{1}{E} \Big(- \nu \big(\sigma_x + \sigma_y \big) \Big) + \alpha \Delta T \\ \epsilon_z &= \Big(\frac{1}{4*10^9} \Big) \big(-0.45* \big(-5*10^6 - 5.45*10^6 \big) \big) + \big(40*10^{-6} \big) *20 = 0.001976 \\ \epsilon_z &= \frac{\Delta z}{z_o} \to \Delta z = \ \epsilon_z * z_o = 0.001176* 0.4 \ m = 0.00079 \ m = 0.79 \ mm \\ Length &= z_o + \Delta z = 0.4 \ m + 0.00079 \ m = 0.40079 \ m \end{split}$$

Problem 2.4 (5 points)

A cube of material is subjected to compressive stresses of σ_0 in the x and y directions and is free to deform in the z direction (no stresses or constraints in the z direction).



Assume small strains and linear materials properties. For different values of the Poisson's ratio, indicate whether the change in volume (ΔV) and the strain in the z direction (ϵ_z) are >0, =0, or <0.

ν	ΔV	ϵ_z
0.5	0	>0
0.2	<0	>0
0	<0	=0
-0.2	<0	<0

Conceptual explanation: the volume change is zero when the Poisson's ratio is 0.5. If the Poisson's ratio is anything other than 0.5, the volume change will be reflective of the stresses: compressive stress results in negative volume change and tensile stress results in positive volume change.

Mathematical explanation:

$$V_{o} = x_{o}y_{o}z_{o}$$

$$V = x_{o}(1 + \epsilon_{x})y_{o}(1 + \epsilon_{y})z_{o}(1 + \epsilon_{z})$$

$$V = x_{o}y_{o}z_{o}(1 + \epsilon_{x} + \epsilon_{y} + \epsilon_{x}\epsilon_{y})(1 + \epsilon_{z})$$

$$V = x_{o}y_{o}z_{o}(1 + \epsilon_{x} + \epsilon_{y} + \epsilon_{x}\epsilon_{y} + \epsilon_{z} + \epsilon_{x}\epsilon_{z} + \epsilon_{y}\epsilon_{z} + \epsilon_{x}\epsilon_{y}\epsilon_{z})$$

When strains are small, $\epsilon_x \epsilon_y$, $\epsilon_x \epsilon_z$, $\epsilon_y \epsilon_z \ll \epsilon_x$, ϵ_y , ϵ_z

$$V \sim x_o y_o z_o \left(1 - 2 \left(\frac{\sigma_o}{E} \right) (1 - \nu) + \left(\frac{\sigma_o}{E} \right) (2\nu) \right) = x_o y_o z_o \left(1 + \left(\frac{\sigma_o}{E} \right) (4\nu - 2) \right)$$

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$$\Delta V = V - V_o = x_o y_o z_o \left(\left(\frac{\sigma_o}{E} \right) (4\nu - 2) \right)$$

Which is 0 when $\nu = 0.5$ and negative for all other values of the Poisson's ratio.

$$\epsilon_z = \left(\frac{1}{E}\right) \left(\sigma_z - \nu \left(\sigma_x + \sigma_y\right)\right) = \left(\frac{1}{E}\right) 2\nu |\sigma_o|$$