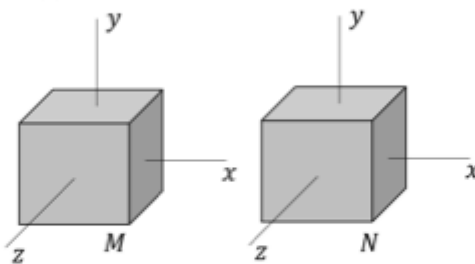
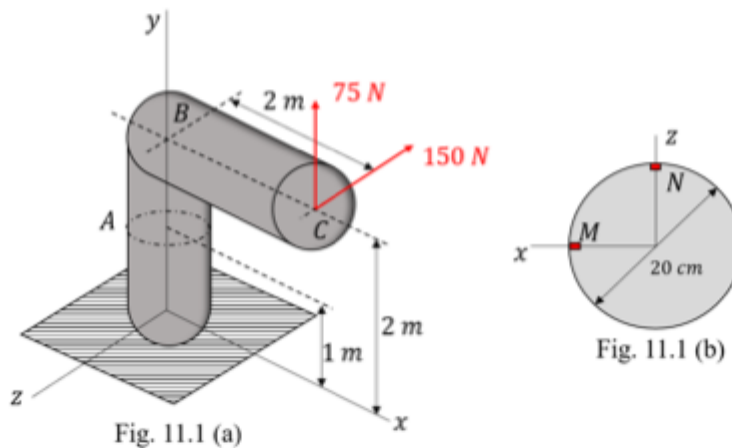


Problem 11.1 (10 Points)

The elbow shown below is fixed to the ground at the center of the coordinate system. Two loads 75 N and 150 N are applied at the free end in the y and z directions, respectively. If the elbow has a circular cross-section with a diameter of 20 cm, find:

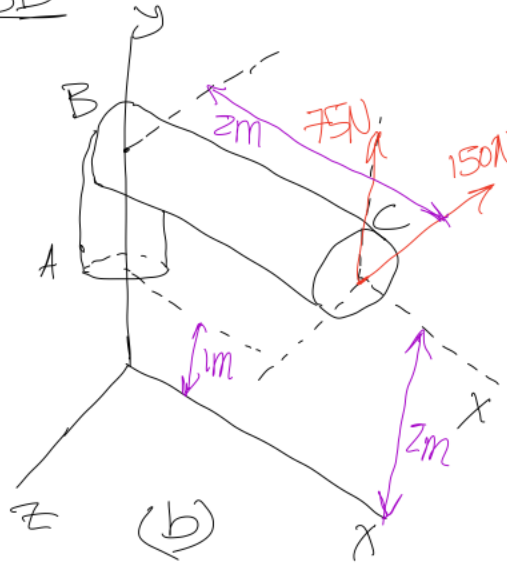
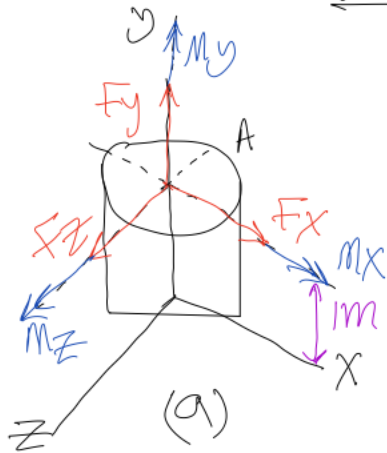
- (a) The internal reactions at a cross-section perpendicular to the y -axis at point A ($y = 1$ m). Classify the forces as either axial or shear forces, and the moments as either bending or torsion.
- (b) The stresses induced (magnitude and direction) in the stress elements M and N on the cross-section at A, shown in Fig. 11.1 (b), due to each reaction calculated in part (a). Use the three-dimensional stress elements in Fig. 11.1 (c).



Solution:

(a)

FBD



$$\vec{r}_{C/A} = z\vec{i} + \vec{j}$$

from (a),

$$\vec{F}_A = F_x\vec{i} + F_y\vec{j} + F_z\vec{k}$$

$$\vec{M}_A = M_x\vec{i} + M_y\vec{j} + M_z\vec{k}$$

From (b),

$$\sum \vec{F} = 0$$

$$-\vec{F}_A + 75\vec{j} - 150\vec{k} = 0$$

$$\therefore \vec{F}_A = 75\vec{j} - 150\vec{k} \text{ (N)}$$

$$\begin{aligned}\sum \vec{M} &= 0 \\ -\vec{M}_A + \vec{r}_{C/A} \times (75\vec{j} - 150\vec{k}) &= 0 \\ \Rightarrow \vec{M}_A &= (2\vec{i} + \vec{j}) \times (75\vec{j} - 150\vec{k}) \\ \therefore \vec{M}_A &= -150\vec{i} + 300\vec{j} + 150\vec{k} \quad (\text{Nm})\end{aligned}$$

	Reaction at A	Type of reaction
1	$75\vec{j}$ N	Axial force
2	$-150\vec{k}$ N	shear force
3	$-150\vec{i}$ Nm	Bending moment
4	$300\vec{j}$ Nm	Torsion
5	$150\vec{k}$ Nm	Bending moment

(b)

For the circular cross section,

$$A = \frac{\pi (0.2)^2}{4} = 0.0314 \text{ m}^2$$

$$I = \frac{\pi (0.2)^4}{64} = 7.854 \times 10^{-5} \text{ m}^4$$

$$J = I_p = \frac{\pi (0.2)^4}{32} = 1.571 \times 10^{-4} \text{ m}^4$$

1. $75\vec{j}$ N (axial force)

$$\sigma_{m,1} = \sigma_{N,1} = \sigma_y = \frac{F_y}{A} = \frac{75}{0.0314} = 2388.53 \text{ Pa}$$

2. $-150 \vec{k}$ N (shear force in z)

$$\tau_{M1} = \tau_{yz} = \frac{4Fz}{3A} = \frac{4(-150)}{3(0.0314)} \\ = -6369.42 \text{ Pa}$$

$$\tau_{N1} = 0$$

3. $-150 \vec{i}$ Nm (Bending moment)

$$\sigma_{N2} = \sigma_y = \left| \frac{M_x R}{I} \right| = \frac{(150)(0.1)}{7.854 \times 10^{-5}} \\ = 1.91 \times 10^5 \text{ Pa}$$

$$\sigma_{M1} = \sigma_y = 0$$

4. $300 \vec{j}$ Nm (Torsion)

$$\tau_{M12} = \tau_{yz} = -\left| \frac{M_y R}{J} \right| = \frac{-300(0.1)}{1.571 \times 10^{-4}} \\ = -1.91 \times 10^5 \text{ Pa}$$

$$\tau_{N12} = \tau_{yx} = \left| \frac{M_y R}{J} \right| = \frac{300(0.1)}{1.571 \times 10^{-4}} = 1.91 \times 10^5 \text{ Pa}$$

5. $150 \vec{k}$ Nm (Bending moment)

$$\sigma_{N13} = \sigma_y = 0$$

$$\sigma_{M3} = \sigma_y = \left| \frac{M_z R}{I} \right| = \frac{(150)(0.1)}{7.854 \times 10^{-5}}$$

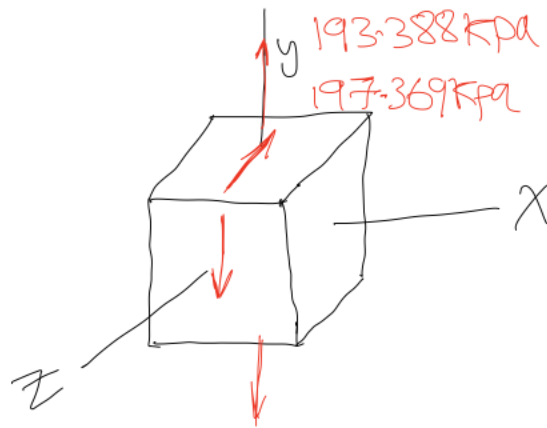
$$= 1.91 \times 10^5 \text{ Pa}$$

In summary,

The state of stress at M:

$$\begin{aligned}\sigma_{M,y} &= \sigma_{M,1} + \sigma_{M,2} + \sigma_{M,3} \\ &= 193.388 \text{ kPa}\end{aligned}$$

$$\tau_{M,yz} = \tau_{M,1} + \tau_{M,2} = -197.369 \text{ kPa}$$



The state of stress at N:

$$\sigma_{N,y} = \sigma_{N,1} + \sigma_{N,2} + \sigma_{N,3} = 193.388 \text{ kPa}$$

$$\tau_{N,yx} = \tau_{N,1} + \tau_{N,2} = 191 \text{ kPa}$$

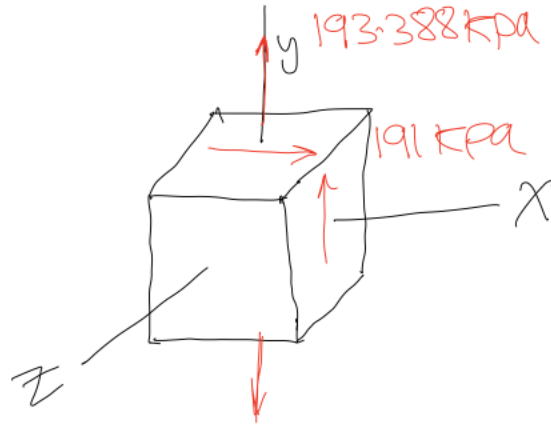


Table method =

	Reaction at A	stress comp. @ "M"	stress comp. @ "N"
1	$F_x = 75\text{N}$	$\sigma_y = \frac{F_x}{A} = 2388.53 \text{ Pa}$	$\sigma_y = \frac{F_x}{A} = 2388.53 \text{ Pa}$
2	$F_z = -150\text{N}$	$\tau_{yz} = \frac{4F_z}{3A} = -636942 \text{ Pa}$	—
3	$M_x = -150\text{Nm}$	—	$\sigma_y = \left \frac{M_x R}{I} \right = 1.91 \times 10^5 \text{ Pa}$
4	$M_y = 300\text{Nm}$	$\tau_{yz} = -\left \frac{M_y R}{J} \right = -1.91 \times 10^5 \text{ Pa}$	$\tau_{yx} = \left \frac{M_y R}{J} \right = 1.91 \times 10^5 \text{ Pa}$
5	$M_z = 150 \text{ N}\cdot\text{m}$	$\sigma_y = \left \frac{M_z R}{I} \right = 1.91 \times 10^5 \text{ Pa}$	—

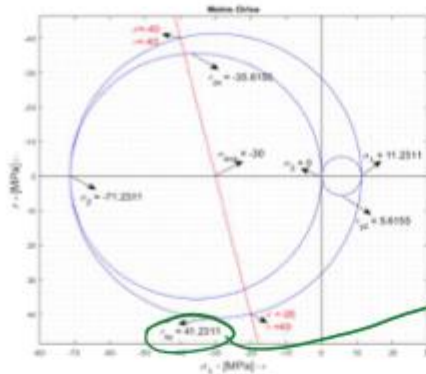
Problem 11.2

a) For fig. 11.1(a)

$$\sigma_x = -40 \text{ MPa}; \quad \sigma_y = -20 \text{ MPa}; \quad \tau_{xy} = -40 \text{ MPa}$$

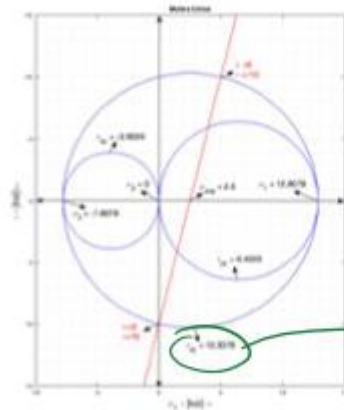
For fig. 11.1(b)

$$\sigma_z = 5 \text{ ksi}; \quad \tau_{xz} = 10 \text{ ksi}$$
$$\sigma_x = 0 \text{ ksi}$$



11. a

$$\tau_{\max, \text{abs.}} = 41.231 \text{ MPa}$$

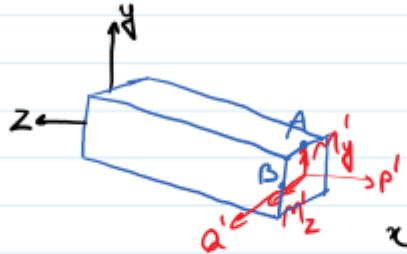


11. b

$$\tau_{\max, \text{abs.}} = 10.31 \text{ ksi}$$

Problem 11.3

a) FBD



Reactions at face AB:

- 1) $P' = P = -10$ kips (Axial force)
- 2) $Q' = Q = 5$ kips (Shear force)
- 3) $M_y' = M_y - Q(5) = 5 - 25 = -20$ kips·in
(Bending moment)
- 4) $M_z' = M_z = 2$ kips·in
(Bending moment)

Geometrical properties of face AB:

$$A = bh = (1)(3) = 3 \text{ in}^2$$

$$I_z = \frac{1}{12}(b)(h)^3 = 2.25 \text{ in}^4$$

$$I_y = \frac{1}{12}(h)(b)^3 = 0.25 \text{ in}^4$$

Stresses caused due to each of the above four reactions:

1) P' :

$$\sigma_{x,A} = \sigma_{x,B} = \frac{P'}{A} = -3.33 \text{ ksi}$$

2) Q' :

$$\tau_{xz,A} = \frac{Q'A^*z^*}{I_y t}; \quad A^* = (3)(0.5) = 1.5 \text{ in}^2$$
$$z^* = 0.25 \text{ in}$$
$$t = 3 \text{ in}$$

$$\Rightarrow \tau_{xz,A} = 2.5 \text{ ksi} ; \tau_{xz,B} = 0 (\because \text{it's at neutral axis})$$

3) M'_y :

$$\sigma_{x,B} = - \left| \frac{M'_y z}{I_y} \right|$$

$$= -40 \text{ ksi} ; z = 0.5 \text{ in}$$

$$\sigma_{x,A} = 0 (\because z = 0)$$

4) M'_z :

$$\sigma_{x,A} = - \left| \frac{M'_z y}{I_z} \right| ; y = 1.5 \text{ in}$$

$$= -1.33 \text{ ksi}$$

$$\sigma_{x,B} = 0 (\because y = 0)$$

\therefore Total Stresses at A:

$$\sigma_{x,A} = -3.33 + 0 - 1.33 = -4.67 \text{ ksi}$$

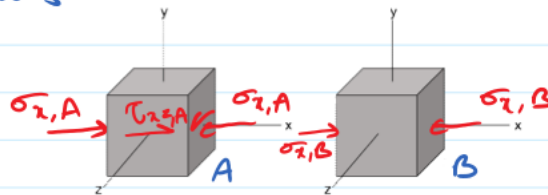
$$\tau_{xz,A} = 2.5 \text{ ksi}$$

Total stresses at B:

$$\sigma_{x,B} = -3.33 - 40 + 0 = -43.33 \text{ ksi}$$

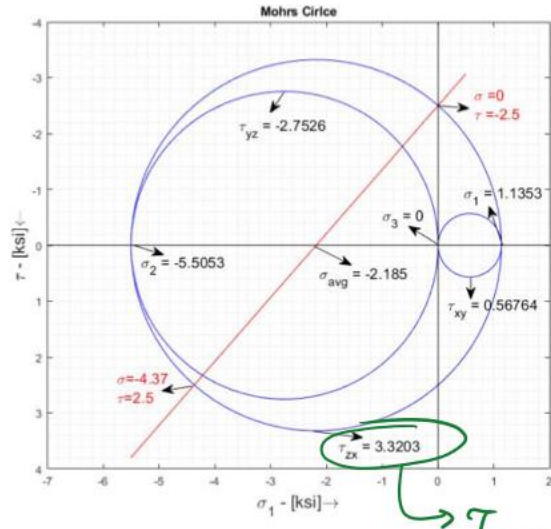
$$\tau_{xz,B} = 0$$

States of stress



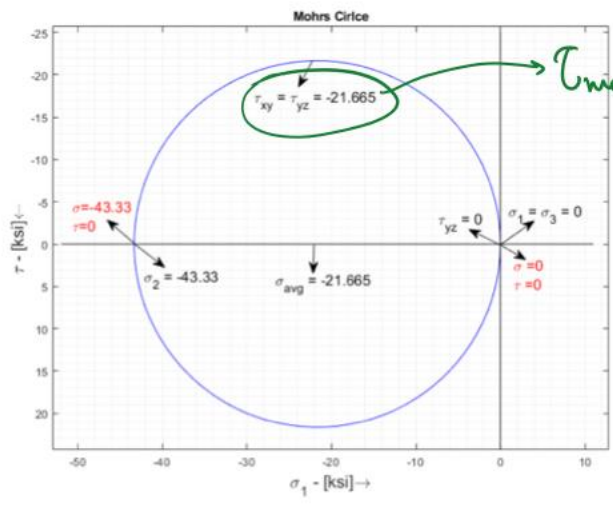
b)

A



$\tau_{max, abs} = 3.32 \text{ ksi}$

B



$\tau_{max, abs} = 21.665 \text{ ksi}$

Problem 4

Critical load:

$$P_{cr} = \frac{\pi^2 E I}{(KL)^2}, \quad I = \frac{\pi R^4}{4}$$

(a) $K = 1$ (pinned-pinned)

$$P_{cr}^{(a)} = \frac{\pi^2 \times E \times \pi R^4}{4 \times (1 \times L)^2} = \frac{1}{4} \left(\frac{\pi^3 E R^4}{L^2} \right)$$

(b) $K = 2$ (fixed-free)

$$P_{cr}^{(b)} = \frac{\pi^2 \times 4E \times \pi \times (R/2)^4}{4 \times (2L)^2} = \frac{1}{64} \left(\frac{\pi^3 E R^4}{L^2} \right)$$

(c) $K = 0.7$ (fixed-pinned)

$$P_{cr}^{(c)} = \frac{\pi \times 3E \times \pi R^4}{4 \times \left(\frac{0.7L}{3}\right)^2} = 13.78 \left(\frac{\pi^3 E R^4}{L^2} \right)$$

(d) $K = 0.5$ (fixed-fixed)

$$P_{cr}^{(d)} = \frac{\pi \times 4E \times \pi (2R)^4}{4 \times (0.5L)^2} = 64 \left(\frac{\pi^3 E R^4}{L^2} \right)$$

$$\therefore P_{cr}^{(d)} > P_{cr}^{(c)} > P_{cr}^{(a)} > P_{cr}^{(b)}$$