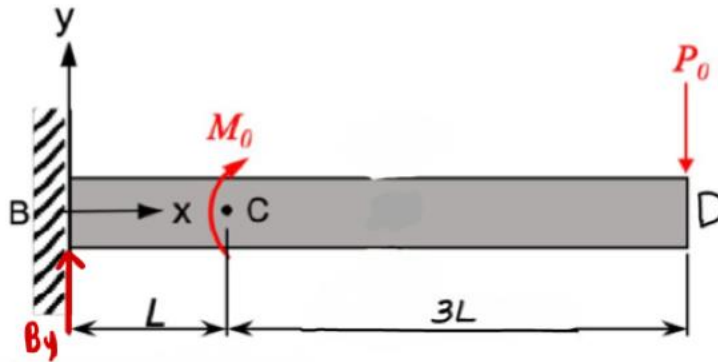


Problem | (10 points) The beam shown below has a Young's modulus E and second area moment of inertia I . Determine the following using the second-order integration method:

- (a) Bending moment $M(x)$ along the length of the beam.
- (b) The slope of the beam $\theta(x)$.
- (c) The deflection of the beam $v(x)$.



$$\sum F_y = 0 : B_y - P_0 = 0 \Rightarrow B_y = P_0 \quad \textcircled{1}$$

$$+\sum \mathcal{M}_B = 0 : -M_0 - P_0(4L) = 0 \Rightarrow M_B = -M_0 - 4P_0L$$

Section BC $0 \leq x \leq L$

$$\Rightarrow M_0 : M_i - M_B - B_y x = 0$$

$$\Rightarrow M_i = M_B + B_y x$$

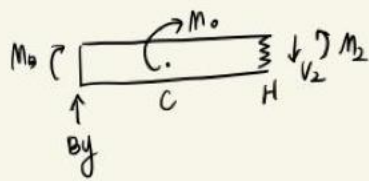
$$\theta_1(x) = \theta_1(0) + \frac{1}{EI} \int_0^x M_i dx$$

$$= \frac{1}{EI} \left(M_B x + \frac{B_y}{2} x^2 \right)$$

$$v_1(x) = v_0(x) + \int_0^x \theta_1(x) dx$$

$$= \frac{1}{EI} \left(\frac{M_B}{2} x^2 + \frac{B_y}{6} x^3 \right)$$

Section CD $L < x \leq 4L$



$$\Rightarrow M_H: M_2 - M_0 - M_B - B_y x = 0$$

$$\Rightarrow M_2 = M_B + M_0 + B_y x$$

$$\theta_2(x) = \theta_1(L) + \frac{1}{EI} \int_L^x M_2 dx$$

$$= \frac{1}{EI} \left(M_B L + \frac{B_y}{2} L^2 \right) + \frac{1}{EI} \left[(M_B + M_0) x + \frac{B_y}{2} x^2 \right]_L^x$$

$$= \frac{1}{EI} \left(M_B L + \frac{B_y}{2} L^2 \right) + \frac{1}{EI} \left\{ (M_B + M_0) x + \frac{B_y}{2} x^2 \right\}$$

$$- \frac{1}{EI} \left\{ (M_B + M_0) L + \frac{B_y}{2} L^2 \right\}$$

$$= \frac{1}{EI} \left\{ (M_B + M_0) x + \frac{B_y}{2} x^2 - M_0 L \right\}$$

$$v_2(x) = v_1(L) + \int_L^x \theta_2(x) dx$$

$$= \frac{1}{EI} \left(\frac{M_B}{2} L^2 + \frac{B_y}{6} L^3 \right) + \frac{1}{EI} \left\{ \frac{1}{2} (M_B + M_0) x^2 + \frac{B_y}{6} x^3 - M_0 L x \right\}$$

$$- \frac{1}{EI} \left\{ \frac{1}{2} (M_B + M_0) L^2 + \frac{B_y}{6} L^3 - M_0 L^2 \right\}$$

$$= \frac{1}{EI} \left\{ (M_B + M_0) \frac{x^2}{2} + \frac{B_y}{6} x^3 - M_0 L x + \frac{M_0 L^2}{2} \right\}$$

Eqn ① & ②

$$\Rightarrow \text{(a) } M(x) = \begin{cases} -M_0 - 4P_0L + P_0x & 0 \leq x \leq L \\ -4P_0L + P_0x & L \leq x \leq 4L \end{cases}$$

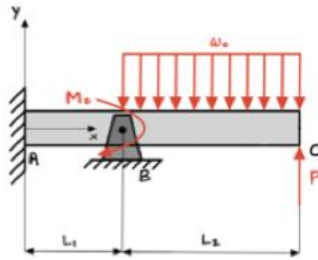
$$\text{(b) } \theta(x) = \begin{cases} \frac{1}{EI} \left\{ (-M_0 - 4P_0L)x + \frac{P_0}{2}x^2 \right\} & 0 \leq x \leq L \\ \frac{1}{EI} \left\{ -4P_0Lx + \frac{P_0}{2}x^2 - P_0L \right\} & L \leq x \leq 4L \end{cases}$$

$$\text{(c) } v(x) = \begin{cases} \frac{1}{EI} \left\{ (-M_0 - 4P_0L)\frac{x^2}{2} + \frac{P_0}{6}x^3 \right\} & 0 \leq x \leq L \\ \frac{1}{EI} \left\{ -2P_0Lx^2 + \frac{P_0}{6}x^3 - M_0Lx + \frac{M_0}{2}L^2 \right\} & L \leq x \leq 4L \end{cases}$$

Problem 2 (10 points)

A cantilever beam ABC with a circular cross section has a roller support at B and is subjected to a uniformly distributed load w_0 between B and C, a concentrated load P at C, and an external couple M_0 at B. Using the superposition principles, determine:

- (a) The reaction at point B.
- (b) The analytical expression for the deflection of the beam v_x .
- (c) The deflection at point C.

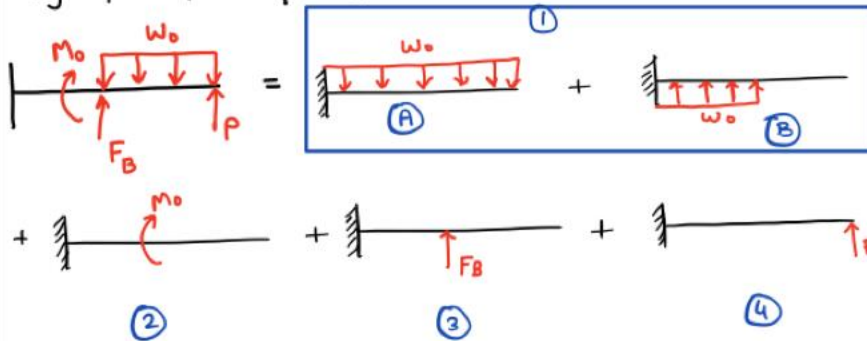


Data: $L_1 = 1$ m, $L_2 = 2$ m, $d = 40$ mm, $E = 200$ GPa, $P = 150$ N, $M_0 = 100$ N.m, and $w_0 = 80$ N/m.

Note: Please refer to the attached table for the necessary beam deflection equations.

By inspection, we can state that beam is **Statically indeterminate.**

By principle of superposition:



Their individual deflections can be estimated from the charts:

①:

$$\rightarrow \textcircled{A}: V_A(x) = -\frac{80x^2}{24EI} (54 - 12x + x^2) \quad 0 \leq x \leq 3$$

$$\textcircled{B}: V_B(x) = \begin{cases} \frac{80x^2}{24EI} (6 - 4x + x^2) & 0 \leq x \leq 1 \\ \frac{80}{24EI} (4x - 1) & 1 \leq x \leq 3 \end{cases}$$

$$\textcircled{2}: V_2(x) = \begin{cases} -\frac{50x^2}{EI} & 0 \leq x \leq 1 \\ -\frac{50(2x-1)}{EI} & 1 \leq x \leq 3 \end{cases}$$

$$\textcircled{3}: V_3(x) = \begin{cases} \frac{F_B x^2}{6EI} (3-x) & 0 \leq x \leq 1 \\ \frac{F_B (3x-1)}{6EI} & 1 \leq x \leq 3 \end{cases}$$

$$\textcircled{4}: V_4(x) = \frac{150x^2}{6EI} (9-x) \quad 0 \leq x \leq 3$$

To get the overall deflection of the beam, we will have to add ①, ②, ③ & ④ together [in their respective domains], and the value will add upto zero at $x=1$.

Now,

$$V_{ABC}(x) = \begin{cases} 0 \leq x \leq 1\text{m} \\ -\frac{80x^2}{24EI}(54 - 12x + x^2) + \frac{80x^2}{24EI}(6 - 4x + x^2) \\ -\frac{50x^2}{EI} + \frac{F_B x^2}{6EI}(3-x) + \frac{150x^2}{6EI}(9-x) \\ 1 \leq x \leq 3\text{m} \\ -\frac{80x^2}{24EI}(54 - 12x + x^2) + \frac{80(4x-1)}{24EI} \\ -\frac{50(2x-1)}{EI} + \frac{F_B(3x-1)}{6EI} + \frac{150x^2}{6EI}(9-x) \end{cases}$$

@

$$V_{ABC}(1\text{m}) = 0$$

$$\therefore -\frac{80}{24EI}(54-12+1) + \frac{80}{24EI}(6-4+1) - \frac{50}{EI}$$

$$+ \frac{F_B}{6EI}(2) + \frac{150}{6EI}(8) = 0$$

$$\Rightarrow -\frac{80}{24}(43) + 10 - 50 + \frac{F_B}{3} + 200 = 0$$

$$F_B = -50 \text{ N}$$

(b)

The analytical expression $V_{ABC}(x)$ can now be modified as follows:

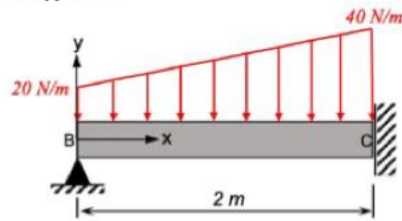
$$V_{ABC}(x) = \begin{cases} 0 \leq x \leq 1 \text{ m} \\ \frac{1}{EI} (10x^3 - 10x^2) \\ 1 \leq x \leq 3 \text{ m} \\ \frac{1}{EI} (-10x^4 + 15x^3 + 45x^2 - \frac{335}{3}x + 55) \end{cases}$$

@ Deflection at C:

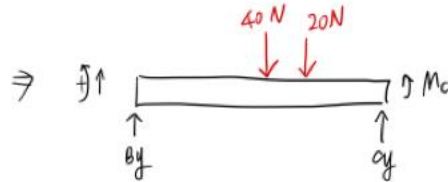
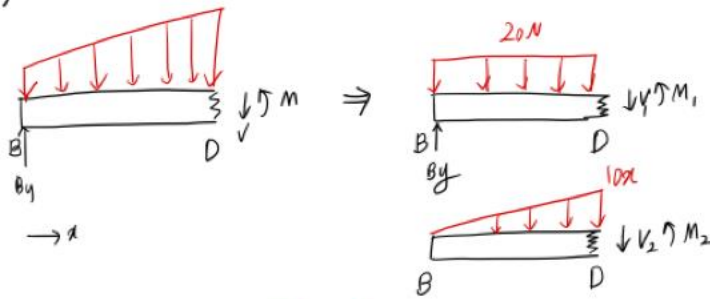
$$V_{ABC}(3 \text{ m}) = \frac{(-10(81) + 15(27) + 45(9) - 335 + 55)}{(200 \text{ GPa}) \left(\frac{\pi}{4} (0.07)^4\right)}$$
$$= 0.0103 \text{ m}$$

Problem 3 (10 points) The beam shown below has a Young's modulus of 50 GPa and has a square cross-section of side 1 cm. Determine the following using the second-order integration method:

- (a) Bending moment $M(x)$ along the length of the beam.
- (b) The slope of the beam $\theta(x)$.
- (c) The deflection of the beam $v(x)$.
- (d) Slope of the beam at support θ_B .



(a)



$$\sum F_y = B_y + C_y - 60 = 0$$

$$B_y + C_y = 60 \quad \text{--- (3)}$$

$$\sum M_c = M_c - B_y(2) + 40(1) + 20\left(\frac{2}{3}\right) = 0$$

$$\Rightarrow M_c - 2B_y = -\frac{160}{3} \quad \text{--- (2)}$$

$$M_D = M_1 + M_2 = \left\{ B y x - 20 x \left(\frac{x}{2} \right) \right\} + \left\{ -5 x^2 \left(\frac{x}{3} \right) \right\}$$

$$M(x) = B y x - 10 x^2 - \frac{5}{3} x^3 \quad (\text{indeterminate beam})$$

$$\begin{aligned} \text{(b) } \theta(x) &= \theta(0) + \frac{1}{EI} \int_0^x M(x) dx \\ &= \theta(0) + \frac{1}{EI} \left[\frac{B y}{2} x^2 - \frac{10}{3} x^3 - \frac{5}{12} x^4 \right] \end{aligned}$$

$$\begin{aligned} \text{(c) } v(x) &= \cancel{v(0)} + \int_0^x \theta(x) dx \\ &= \theta(0) x + \frac{1}{EI} \left[\frac{B y}{6} x^3 - \frac{5}{6} x^4 - \frac{1}{12} x^5 \right] \end{aligned}$$

B.C's

$$v(2) = 0$$

$$\Rightarrow v(2) = 2\theta(0) + \frac{1}{EI} \left[\frac{4}{3} B y - \frac{40}{3} - \frac{8}{3} \right]$$

$$\Rightarrow \theta(0) = -\frac{1}{EI} \left(\frac{2}{3} B y - 8 \right)$$

$$\theta(2) = 0$$

$$\Rightarrow \theta(2) = \theta(0) + \frac{1}{EI} \left(2B y - \frac{100}{3} \right)$$

$$= -\frac{1}{EI} \left(\frac{2}{3} B y - 8 \right) + \frac{1}{EI} \left(2B y - \frac{100}{3} \right)$$

$$\Rightarrow B y = 19 \text{ N} \Rightarrow C_v = 41 \text{ N}, M_c = -15.33 \text{ N}\cdot\text{m}$$

$$I = \frac{b^4}{12} = \frac{(0.01)^4}{12} \Rightarrow EI = 41.67 \text{ N}\cdot\text{m}^2$$

$$\theta(0) = -\frac{1}{41.67} \left(\frac{14}{3}\right) = -0.112 \text{ rad} \approx -6.42^\circ$$

$$\Rightarrow M(x) = 19x - 10x^2 - \frac{5}{3}x^3 \quad (\text{N}\cdot\text{m})$$

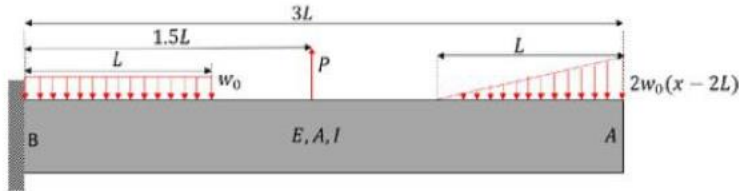
$$\theta(x) = -0.112 + \frac{1}{41.67} \left\{ \frac{19}{2}x^2 - \frac{10}{3}x^3 - \frac{5}{12}x^4 \right\} \text{ rad}$$

$$V(x) = -0.112x + \frac{1}{41.67} \left\{ \frac{19}{6}x^3 - \frac{5}{6}x^4 - \frac{1}{12}x^5 \right\} \text{ m}$$

$$(d) \quad \theta_B = \theta(0) = -0.112 \text{ rad} = -6.42^\circ$$

Problem 4 (5 points)

1. Consider the cantilevered beam with distributed loads as shown below



Select the general form that the equation for flexural energy due to bending will take (note that $f_i(x)$ represents some continuous function $M(x)/(2EI)$ for the length of integration):

(a) $U = \int_0^L f_1(x)dx + \int_L^{2L} f_2(x)dx + \int_{2L}^{3L} f_3(x)dx$

(b) $U = \int_0^L f_1(x)dx + \int_L^{1.5L} f_2(x)dx + \int_{1.5L}^{3L} f_3(x)dx$

(c) $U = \int_0^L f_1(x)dx + \int_L^{1.5L} f_2(x)dx + \int_{1.5L}^{2L} f_3(x)dx + \int_{2L}^{3L} f_4(x)dx$

(d) $U = \int_0^L f_1(x)dx + \int_0^{1.5L} f_2(x)dx + \int_0^{2L} f_3(x)dx + \int_0^{3L} f_4(x)dx$

$\{0 < x < L\}$, $\{L < x < 1.5L\}$, $\{1.5L < x < 2L\}$, and $\{2L < x < 3L\}$
 each have unique equations for bending moment. Integration is performed section by section so that the sum of integrals spans the region of interest.