

Problem 1 (10 points):

The beam AD is fixed to a rigid wall at A and is supported by props at B and C as shown in *figure 1*. In sections AB and BC, the flexural rigidity is EI , but in section CD the flexural rigidity is $2EI$. The beam supports a linearly distributed load over span BC.

Use Castigliano's Second Theorem (neglecting shear energy) to determine:

1. Reactions at end A.
2. Slope θ of the beam at the support C.

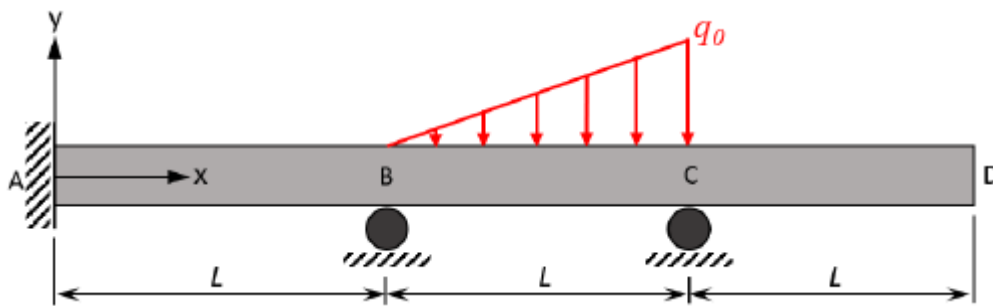
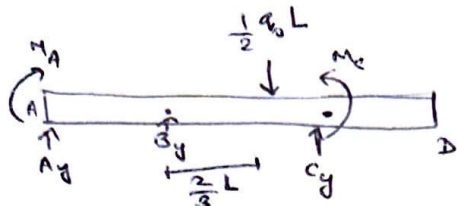


Figure 1: Beam loading for Problem 1

Problem 1

FBD:



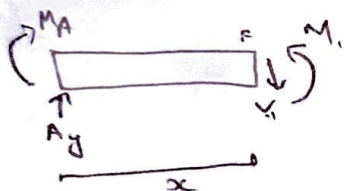
$$\sum F_y = A_y + B_y + C_y - \frac{q_0 L}{2} = 0 \Rightarrow A_y + B_y + C_y = \frac{q_0 L}{2}$$

$$\sum M_A = B_y L - M_A + C_y (2L) + M_c - \frac{q_0 L}{2} \times \frac{5L}{3} = 0$$

$$M_A = M_c + B_y L + C_y 2L - \frac{5q_0 L^2}{6}$$

Indeterminate problem

Section AB ($0 \leq x \leq L$)



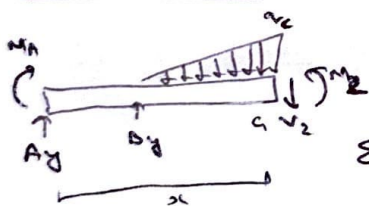
$$\sum M_F = M_1 - A_y x - M_A = 0$$

$$M_1 = M_A + A_y x$$

$$= M_c + B_y L + C_y 2L - \frac{5q_0 L^2}{6} + \left(\frac{q_0 L}{2} - B_y - C_y\right)x$$

$$M_1 = M_c + \frac{q_0 L}{2} \left(x - \frac{5L}{3}\right) + B_y(L-x) + C_y(2L-x)$$

Section BC ($L < x \leq 2L$)



$$q_c = \frac{q_0(x-L)}{L}$$

$$\sum M_C = M_2 - A_y x - B_y(x-L) + \frac{q_c}{2}(x-L)\left(\frac{x-L}{3}\right) = 0$$

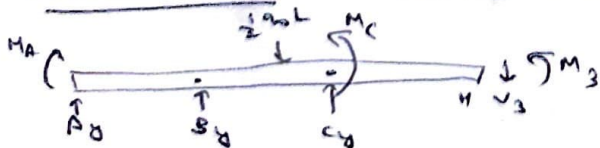
$$M_2 = M_A + A_y x + B_y(x-L) - \frac{q_0(x-L)^3}{6L}$$

$$= M_c + \frac{q_0 L}{2} \left(x - \frac{5L}{3}\right) + B_y(L-x) + C_y(2L-x) + B_y(x-L) - \frac{q_0(x-L)^3}{6L}$$

$$M_2 = M_c + C_y(2L-x) - \frac{q_0(x-L)^3}{6L} + \frac{q_0 L}{2} \left(x - \frac{5L}{3}\right)$$

$$= M_c + C_y(2L-x) + \frac{q_0}{2} \left(x - \frac{5L}{3} - \frac{(x-L)^3}{3L}\right)$$

Section CD ($2L < x \leq 3L$)



$$\sum M_H = 0 = M_3 - M_A - A_y x - B_y(x-L) + \frac{1}{2}q_0 L \left(x - \frac{5L}{3}\right) - C_y(x-2L) + M_c = 0$$

$$M_3 = M_A - M_c + A_y x + B_y(x-L) - \frac{q_0 L}{2} \left(x - \frac{5L}{3}\right) + C_y(x-2L)$$

$$M_3 = M_c + \frac{q_0 L}{2} \left(x - \frac{5L}{3} \right) + B_y(L-x) - M_c + B_y(2L) + C_y(2L-x) + C_y(x-2L) - \frac{q_0 L}{2} \left(x - \frac{5L}{3} \right)$$

$$M_3 = 0$$

$$U = U_1 + U_2 + U_3$$

$$= \frac{1}{2EI} \int_0^L M_1^2 dx + \frac{2L}{EI} \int_0^L M_2^2 dx + \frac{3L}{EI} \int_{2L}^0 M_3^2 dx$$

Now, B_y and C_y are redundant forces

$$\therefore \frac{\partial U}{\partial B_y} = 0 = \frac{1}{2EI} \int_0^L \left\{ M_c + \frac{q_0 L}{2} \left(x - \frac{5L}{3} \right) + B_y(L-x) + C_y(2L-x) \right\} (L-x) dx + 0$$

$$\Rightarrow \int_0^L \left[\frac{q_0 L}{2} \left(x - \frac{5L}{3} \right) (L-x) + B_y(L^2 + 2Lx - 2Lx) + C_y(2L-x)(L-x) \right] dx = 0$$

$$\int_0^L \left[\frac{q_0 L}{6} (8xL - 3x^2 - 5L^2) + x^2(B_y + C_y) - xL(2B_y + 3C_y) + L^2(B_y + 2C_y) \right] dx = 0$$

$$\int_0^L \left[x^2(B_y + C_y - \frac{q_0 L}{2}) - Lx(2B_y + 3C_y - \frac{4}{3} q_0 L) + L^2(B_y + 2C_y - \frac{5}{6} q_0 L) \right] dx = 0$$

$$\frac{L^3}{3} (B_y + C_y - \frac{q_0 L}{2}) - \frac{L^3}{2} (2B_y + 3C_y - \frac{4}{3} q_0 L) + L^3 (B_y + 2C_y - \frac{5}{6} q_0 L) = 0$$

$$\frac{B_y}{3} + \frac{5}{6} C_y - \frac{q_0 L}{3} = 0$$

$$\frac{\partial U}{\partial C_y} = 0 = \frac{1}{2EI} \int_0^L \left\{ M_c + \frac{q_0 L}{2} \left(x - \frac{5L}{3} \right) + B_y(L-x) + C_y(2L-x) \right\} (2L-x) dx$$

$$+ \frac{1}{2EI} \int_{2L}^0 \left\{ M_c + C_y(2L-x) + \frac{q_0 L}{2} \left(xL - \frac{5L^2}{3} - \frac{(x-L)^2}{3L} \right) \right\} (2L-x) dx$$

$$= \int_0^L \left(\frac{q_0 L}{2} (2Lx - \frac{18L^2}{3} - x^2 + \frac{5Lx}{3}) + B_y(2L^2 - xL + 2Lx + x^2) + C_y(4L^2 + x^2 - 4xL) \right) dx$$

$$+ \int_{2L}^0 \left(C_y(4L^2 + x^2 - 4xL) + \frac{q_0 L}{2} \left(2xL - x^2L - \frac{5L^3}{3} + \frac{5L^2x}{3} - \frac{2}{3L} (x^3 - L^3 - 3x^2L + 3xL^2) \right) (2L-x) \right) dx$$

$$= \left[(2B_y L^2 + 4C_y L^2 - \frac{5q_0 L^3}{2}) x - \left(\frac{3}{2} B_y L + 2C_y L - \frac{11}{12} q_0 L^2 \right) x^2 + \frac{x^3}{3} (B_y + C_y - \frac{q_0 L}{2}) \right] \Big|_0^L$$

$$+ \left[(4C_y L^2 - \frac{4}{3} L^3 q_0) x - (2C_y L - \frac{q_0 L^2}{3}) x^2 + \left(\frac{C_y}{3} + \frac{q_0 L}{3} \right) x^3 - \frac{5q_0 x^4}{24} + \frac{q_0 x^5}{30L} \right] \Big|_{2L}^0 = 0$$

$$\left(\frac{5B_y L^3}{6} + \frac{7C_y L^3}{3} - \frac{11q_0 L^4}{12} \right) + \left(\frac{8C_y L^3}{3} - \frac{14q_0 L^4}{15} - \frac{7C_y L^3}{3} + \frac{101q_0 L^4}{120} \right) = 0$$

$$100B_y + 320C_y - 121q_0 L = 0$$

⇒ Solving ④ equations and ⑤ unknowns,

$$A_y = -0.05q_0 L \quad B_y = 0.25q_0 L$$

$$C_y = 0.3q_0 L \quad M_A = -0.2833q_0 L^2$$

$$2) \quad \theta_c = \frac{\partial U}{\partial M_c} \Big|_{M_c=0}$$

$$M_1 = M_c - q_0(0.2833L^2 + 0.05Lx)$$

$$M_2 = M_c - q_0(0.0667L^2 + 0.3Lx - 0.5x^2 + 0.1667\frac{x^3}{L})$$

$$\frac{\partial U}{\partial M_c} = \frac{1}{2EI} \left[\int_0^L 2(M_c - q_0(0.2833L^2 + 0.05Lx)) dx + \int_0^{2L} 2(M_c - q_0(0.0667L^2 + 0.3Lx - 0.5x^2 + 0.1667\frac{x^3}{L})) dx \right]$$

$$\theta_c = 0.0248 \frac{q_0 L^3}{EI}$$

Problem 2 (10 points):

A simply-supported beam ABC is subjected to a vertical load P at C . The cross section of the beam is shown in *figure 2*, and given that $P = 1000\text{ N}$ and $L = 2\text{ m}$. The elastic and shear modulus of the material are 200 GPa and 75 GPa , respectively. The form factor for a square cross section is $f_s = 6/5$.

- Compare the flexural energy and the shear energy due to bending.
- Use Castigliano's second theorem to determine the vertical deflection of the end C including the strain energy due to shear.

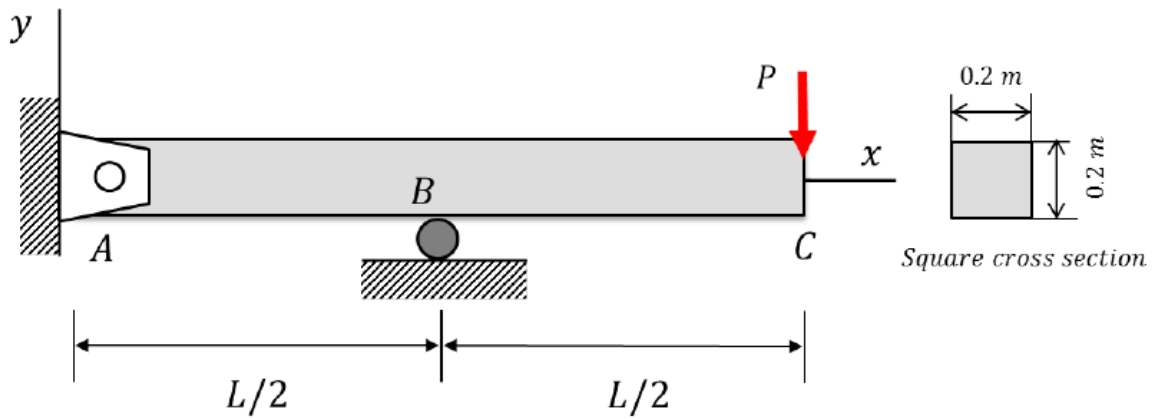
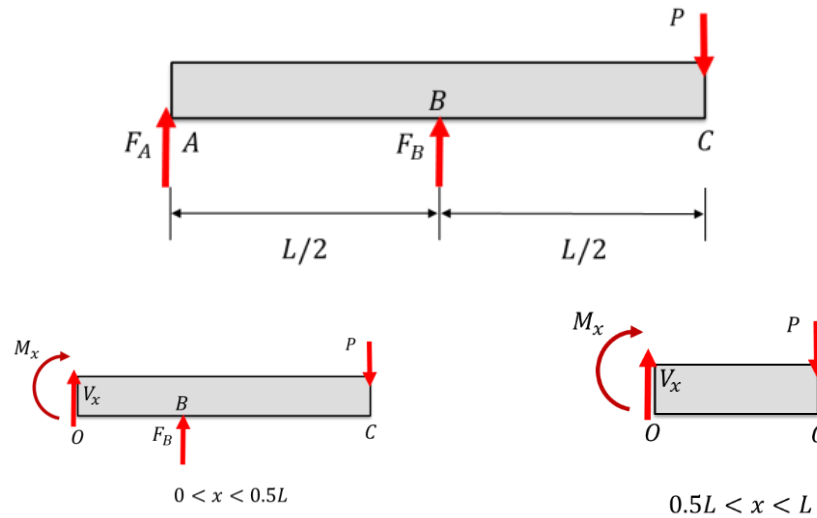


Figure 2: Beam ABC and its cross-section

SOLUTION

a)



FBD analysis over the whole beam,

$$\Sigma M_A = F_B \left(\frac{L}{2} \right) - PL = 0 \Rightarrow F_B = 2P$$

$0 < x < 0.5L$:

$$\Sigma F_y = V_{1x} - P + F_B = 0 \Rightarrow V_{1x} = -P$$

$$\Sigma M_o = -M_{1x} - P(L - x) + F_B \left(\frac{L}{2} - x \right) = 0 \Rightarrow M_{1x} = -Px$$

$0.5L < x < L$:

$$\Sigma F_y = V_{2x} - P = 0 \Rightarrow V_{2x} = P$$

$$\Sigma M_o = -M_{2x} - P(L - x) = 0 \Rightarrow M_{2x} = P(x - L)$$

Flexural energy due to bending,

$$U_{flexural} = \int_0^{0.5L} \frac{M_{1x}^2 dx}{2EI} + \int_{0.5L}^L \frac{M_{2x}^2 dx}{2EI} = \frac{1}{2} \int_0^{0.5L} \frac{P^2 x^2 dx}{EI} + \frac{1}{2} \int_{0.5L}^L \frac{P^2 (x - L)^2 dx}{EI} = \frac{P^2 L^3}{24EI}$$

Shear energy due to bending,

$$U_{shear} = \int_0^{0.5L} \frac{f_s V_{1x}^2 dx}{2GA} + \int_{0.5L}^L \frac{f_s V_{2x}^2 dx}{2GA} = \int_0^L \frac{f_s P^2 dx}{2GA} = \frac{f_s P^2 L}{2GA}$$

$$R = \frac{U_{flexural}}{U_{shear}} = \frac{\frac{P^2 L^3}{24EI}}{\frac{f_s P^2 L}{2GA}} = \frac{G}{f_s E} \left(\frac{L}{d}\right)^2 = \frac{75}{6(200)} \left(\frac{2}{0.2}\right)^2 = 31.25$$

b)

$$U_{total} = U_{flexural} + U_{shear} = 32.25 U_{shear} = 32.25 \frac{f_s P^2 L}{2GA} = 1.29 \times 10^{-2} J$$

Using work-energy principle,

$$U_{total} = \frac{1}{2} P v_c$$

$$v_c = \frac{2U_{total}}{P} = 2.58 \times 10^{-5} m$$

Using Castigliano's Theorem,

$$v_B = \frac{\partial U_{total}}{\partial P} = 32.25 \frac{f_s P L}{GA} = 2.58 \times 10^{-5} m$$

(Either work-energy principle or Castigliano's Theorem is OK)

Problem 3 (10 points):

Figure 3 shows a cantilevered beam (ABC) with modulus E and moment of inertia I . A distributed load of w_0 spans $1/3^{\text{rd}}$ of its length. The beam is simply supported at point B. Use Castigliano's theorem to determine the reactions at A and B. You can neglect the shear energy due to bending.

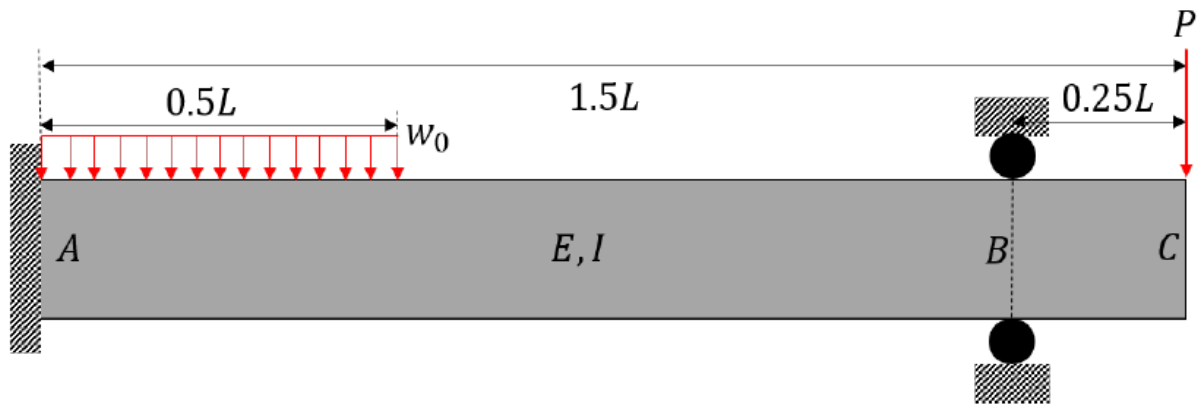
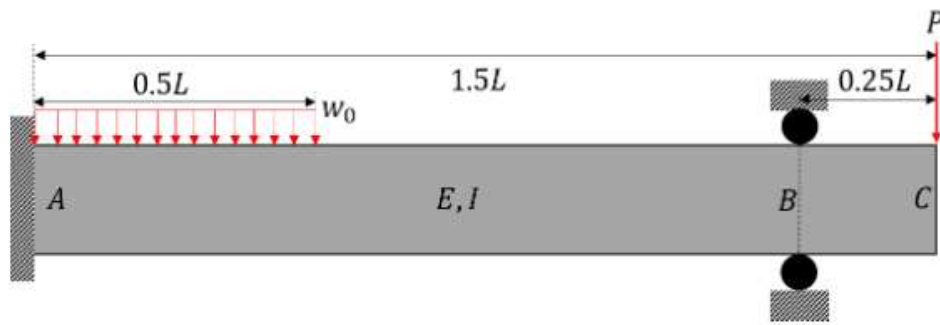
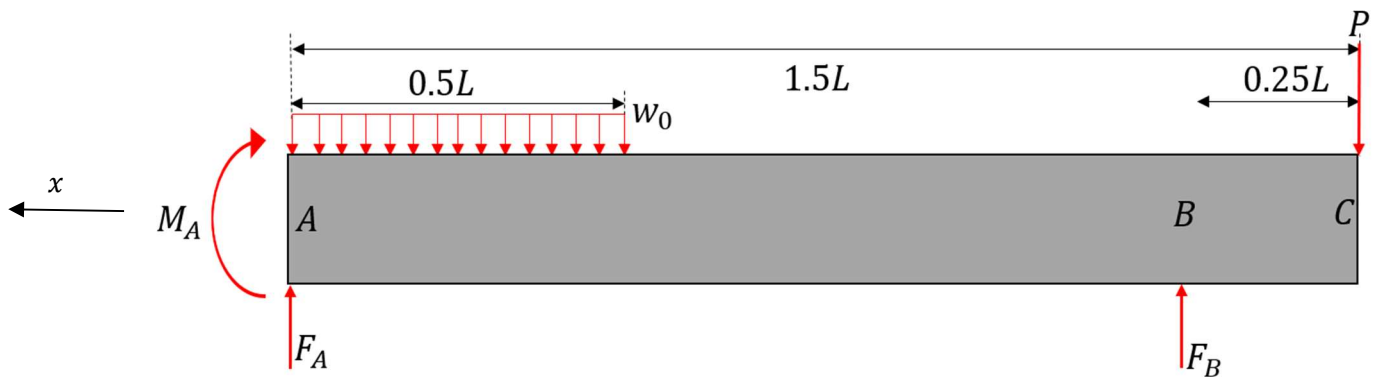


Figure 3: Cantilever beam loading for Problem 3



Solution:



2 pt.

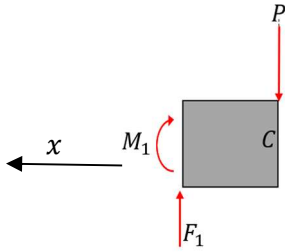
For all FBDs

Equilibrium: Beam is statically indeterminate. 1 pt.

$$\sum M_A = M_A - \frac{1}{2} w_0 L \left(\frac{L}{2} \right) + \frac{5}{4} B_y L - \frac{3}{2} PL$$

$$M_A - \frac{1}{4} w_0 L^2 + \frac{5}{4} B_y L - \frac{3}{2} PL = 0$$

$$\sum F_y = A_y + B_y - P - \frac{1}{2} w_0 L = 0$$



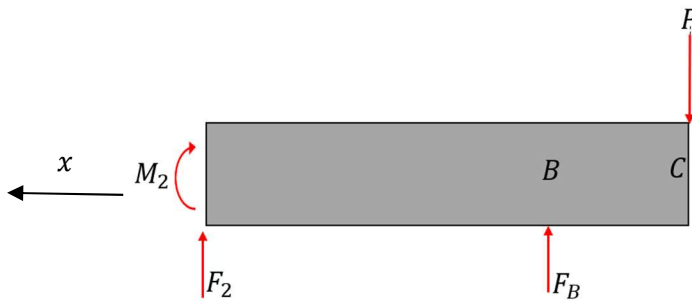
Making a cut at section $0 < x < \frac{L}{4}$ we get:

$$\sum M_{cut} = 0$$

$$-M_1 - Px = 0$$

1 pt.

$$M_1(x) = -Px, \quad \frac{\delta M_1}{\delta B_y} = 0$$



Making a cut at section $\frac{L}{4} < x < L$ we get:

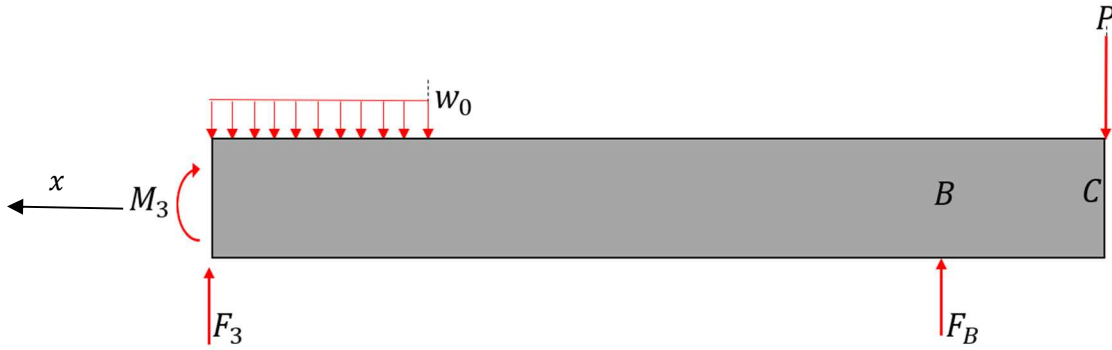
$$\sum M_{cut} = 0$$

$$-M_2 + B_y \left(x - \frac{L}{4} \right) - Px = 0$$

1 pt.

$$M_2(x) = B_y \left(x - \frac{L}{4} \right) - Px, \quad \frac{\delta M_2}{\delta B_y} = x - \frac{L}{4}$$

1 pt. for 3 correct cuts



Making a cut at section $L < x < \frac{3}{2}L$ we get:

$$\Sigma M_{cut} = 0$$

$$-M_3 - \frac{1}{2}w_0(x-L)^2 + B_y\left(x - \frac{L}{4}\right) - Px = 0 \quad 1 \text{ pt.}$$

$$M_3(x) = -\frac{1}{2}w_0(x-L)^2 + B_y\left(x - \frac{L}{4}\right) - Px,$$

$$\frac{\delta M_3}{\delta B_y} = x - \frac{L}{4}$$

$$\text{Total strain energy } = U = \int_0^{\frac{L}{4}} \frac{M_1^2}{2EI} dx + \int_{\frac{L}{4}}^L \frac{M_2^2}{2EI} dx + \int_L^{\frac{3}{2}L} \frac{M_3^2}{2EI} dx \quad 1 \text{ pt.}$$

$$\frac{\delta U}{\delta B_y} = 0 + \int_{\frac{L}{4}}^L \frac{M_2}{EI} \frac{\delta M_2}{\delta B_y} dx + \int_L^{\frac{3}{2}L} \frac{M_3}{EI} \frac{\delta M_3}{\delta B_y} dx = 0 \quad 1 \text{ pt.}$$

$$\frac{1}{EI} \int_{\frac{L}{4}}^L \left(B_y \left(x - \frac{L}{4} \right) - Px \right) \left(x - \frac{L}{4} \right) dx + \frac{1}{EI} \int_L^{\frac{3}{2}L} \left(-\frac{1}{2}w_0(x-L)^2 + B_y \left(x - \frac{L}{4} \right) - Px \right) \left(x - \frac{L}{4} \right) dx = 0$$

$$\frac{9L^3}{128} (2B_y - 3P) + \frac{L^3}{384} (196B_y - 9Lw_0 - 244P) = 0$$

$$\frac{128}{192} B_y = \frac{3}{128} w_0 L + \frac{325}{384} P$$

$$\boxed{B_y = \frac{9}{256} w_0 L + \frac{325}{256} P} \quad 1 \text{ pt.}$$

Optional solutions for A_y and M_a

$$A_y = P + w_oL - B_y = P + w_oL - \frac{9}{256}w_oL - \frac{325}{256}P$$

$$\boxed{A_y = \frac{247}{256}w_oL - \frac{69}{256}P}$$

$$M_A = \frac{1}{4}w_oL^2 - \frac{5}{4}B_yL + \frac{3}{2}PL = \frac{1}{4}w_oL^2 - \frac{45}{1024}w_oL^2 - \frac{1625}{1024}PL + \frac{3}{2}PL$$

$$\boxed{M_A = \frac{211}{1024}w_oL^2 - \frac{89}{1024}PL}$$

Problem 4 (5 points):

Considering all the contributions to strain energy, the total strain energy for the figure 4.1 will have how many non-zero terms?

- a. 3
- b. 5
- c. 6
- d. 7

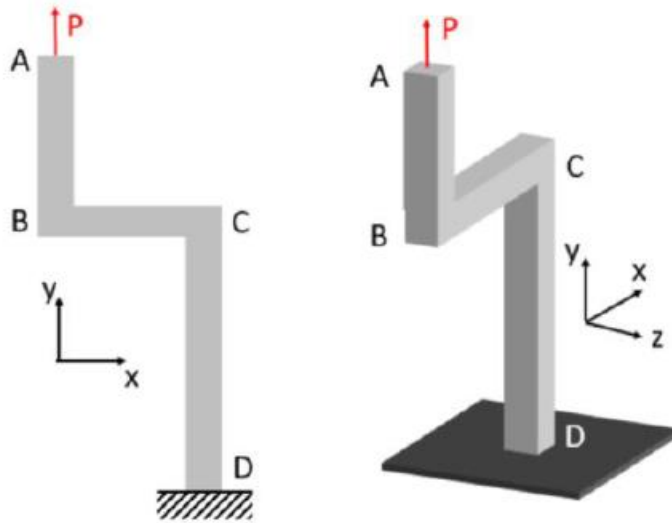
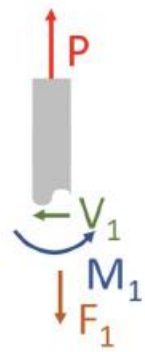
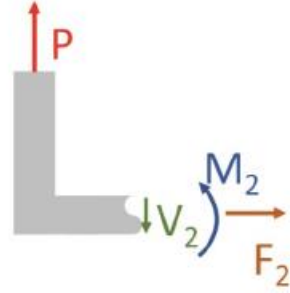


Figure 4: Loading of 3D structure for Problem 4

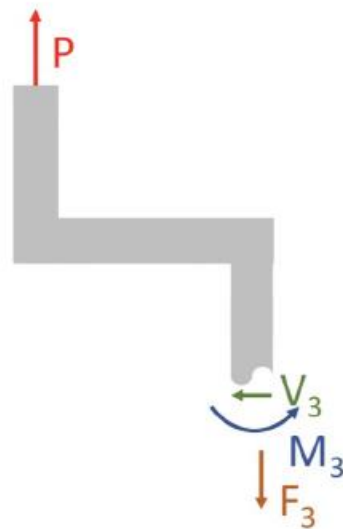
Solution:



$$\begin{aligned} F_1 &= P \\ V_1 &= 0 \\ M_1 &= 0 \end{aligned}$$



$$\begin{aligned} F_2 &= 0 \\ V_2 &= P \\ M_2 &= Px \end{aligned}$$



$$\begin{aligned} F_3 &= P \\ V_3 &= 0 \\ M_3 &= PL_2 \end{aligned}$$

$$U = \frac{1}{2EA} \int_0^{L_1} F_1^2 dy + \frac{f_s}{2GA} \int_0^{L_2} V_2^2 dx + \frac{1}{2EI} \int_0^{L_2} M_2^2 dx + \frac{1}{2EA} \int_0^{L_3} F_3^2 dy + \frac{1}{2EI} \int_0^{L_3} M_3^2 dy$$

→ 5 non-zero terms